Weston Building Committee
Special Meeting Agenda
June 14, 2022 at 7:00 PM
Meeting to be held in the Town Hall Commission Room

1) Call to order
2) Discussion/decision on Town Hall basement records room project
3) Discussion/decision on Senior Center bathroom renovations
4) Approval of Minutes of prior meeting
5) Adjournment

Item 2

KOHLER RONAN, LLC

171 Madison Avenue
New York, NY 10016
T (212) 695-2422
F (212) 695-2423

93 Lake Avenue
Danbury, CT 06810
T (203) 778-1017
F (203) 778-1018

kohlerronan.com

Rory S. Ronan, PE
Craig F. Razza, PE
Joseph V. Lembo, PE
Steven V. Lembo, PE
Erik D. Bodelsen, PE
Talya Santillan, PE

April 21, 2022

via email: jluiz@westonct.gov

Jonathan Luiz
Weston Town Administrator
56 Northfield Road
Weston, CT 06883

Re: Town of Weston – Basement Record
KR Proposal#: OP220008

Dear Jonathan,

Thank you for giving us the opportunity to submit a proposal to provide engineering services for the Basement Record project.

Enclosed is an agreement that reflects our understanding of the project requirements.
Please review the agreement and if it is acceptable, email or mail one copy with your signature.

We look forward to working with you on this project.

Sincerely,

Craig Razza, P.E.

Enclosure

KOHLER RONAN, LLC

[illegible] Madison Avenue
New York, NY 10016
T (212) 695-2422
F (212) 695-2420

93 Lake Avenue
Danbury, CT 06810
T (203) 778-1017
F (203) 778-1018

kohlerronan.com

Rory S. Ronan, PE
Craig F. Razza, PE
Joseph V. Lambo, PE
Steven V. Lambo, PE
Fred D. Donaldson, PE
Tanya Santillan, PE

Proposal for Engineering Services
Town of Weston
Basement Record
Weston, CT
April 21, 2022

This proposal outlines the scope of services to provide Mechanical, Electrical, Plumbing engineering services to produce Construction documents and Contract Administration services. The scope of engineering services for this project is as follows:

Definitions:

Client:	Town of Weston 56 Northfield Road Weston, CT 06883
Project:	Basement Record

Project Understanding:

The project consists of converting an approximately 2,500 s.f. crawl space into a high density record storage room. It is anticipated that there are no unground utilities under the slab that is being removed and lowered to create the storage space. Any uncovered utilities or elements requiring relocation or design are not part of this proposal. The majority of the piping at the ceiling is anticipated to remain and the shelving system design to fit below the piping systems.

The scope of the proposed project shall include the following systems:

HVAC:

- Mini split type air system to provide heating and cooling.
- Ventilation shall be provided through and exterior wall vent.
- Specify insulation to be added to existing heating piping to remain.
- A standalone dehumidifier is proposed to control moisture in the storage space.
- Controls to be stand alone.

Electrical:

- Power distribution in support of HVAC equipment.
- Power distribution as required by the architectural program.
- Circuiting of interior emergency lighting fixtures.
- Circuiting of interior lighting fixtures where indicated on architectural plans.
- Modification to the existing fire alarm system to provide annunciation in the room. Existing smoke detectors to remain.
- Existing IT cabling is anticipated to remain and be resupported where in conflict with the shelving system.

KOHLER RONAN, LLC

Plumbing:

- Modifications to existing plumbing systems to accommodate programming and high density storage system.
- Addition of miscellaneous supports for sanitary piping supported off the floor.

Cost Estimate (Apex Construction):

- Provide opinion of probable cost at prepared by a professional cost estimator.

Scope of Services:

General description: This proposal is based on engineering design services for Mechanical, Electrical, Plumbing (MEP) systems required for the project scope as indicated to accommodate the architectural program at the time of this proposal. All systems designs will be based on Building Code requirements pertinent to the systems designed. All Engineering Services performed will be conducted in accordance with the normal standard of care.

The format of the final product will be one (1) set of drawing plots produced in (AutoCAD) of Mechanical, Electrical, Plumbing work as listed within this proposal. Specifications will be produced on the drawings.

All work will be indicated on architectural backgrounds, reflected ceiling plans, and title blocks as produced (in AutoCAD format) and provided by the Architect/Client including all subsequent refinements.

Construction Documents:

1. Participate in one (1) meeting to coordinate HVAC, Electrical, Plumbing building systems with the Owner.

2. Specify and indicate HVAC systems as indicated by the proposed project scope including plan distribution layouts, mechanical system equipment, diffusers, ductwork, piping systems, schedules, and details sufficient for a competitive contractor bid.

3. Specify and indicate electrical systems as indicated by the proposed project scope including plan distribution, circuiting, lighting, electrical system equipment, panel schedules, and details sufficient for a competitive contractor bid.

4. Specify and indicate plumbing systems as indicated by the proposed project scope including plan distribution layouts of piping systems, plumbing system equipment, schedules, and details sufficient for a competitive contractor bid.

5. Coordinate layout of Mechanical, Electrical and Plumbing systems in conjunction with the architecture based on requirements as determined by the project scope.

6. Specify and indicate lighting based upon architectural ceiling plans.

KOHLER RONAN, LLC

17E Madison Avenue
New York, NY 10016
T (212) 696-2422
F (212) 696-2423

95 Lake Avenue
Danbury, CT 06810
T (203) 778-1017
F (203) 778-1019

kohlerronan.com

Rory S. Ronan, PE
Craig F. Pezza, PE
Joseph V. Lembo, PE
Steven K. Lemos, PE
Erik D. Soderberg, PE
Tolga Soydan, PE

7. Specify and design fire alarm system modifications in support of general program requirements.

8. Prepare a cost estimate as provided by a professional cost estimator at 75% set of CD documents.

Basic Construction Administration Services (Not included):

1. Respond to "Requests for Information" and issue clarifications as required.

2. Included within this proposal is the review of contractor submittals for compliance with the prepared MEP construction documents. Up to 2 site visits are allocated to observe compliance with the construction documents and attend to construction meetings. Additional site visits for observations and or meetings can be provided upon request as an additional service. Attendance at weekly project or coordination meetings during construction are not included and can be provided upon request as an additional service.

3. Conduct up to 1 site visit to prepare final punch list. Additional site visits, if requested to finalize punch list and/or to close out open punch list items can be provided as an additional service.

Work Not Included:

1. Bidding Assist Services.

2. Basic Construction Administration Services but can be added as an hourly service.

3. Fire Protection and Technology design services.

4. Preparation of any certificates, forms, applications, rebates associated with lighting design is not included as part of this proposal.

5. Acoustical Design shall be by the Project Acoustical Consultant. It is the responsibility of the Architect/Owner to retain an acoustical consultant to confirm project meets local noise standards and to provide sound attenuation strategies as necessary to meet all project noise requirements.

6. Plumbing work beyond 5'-0" outside the building.

7. Services associated with Filing, Permits, Licenses, Controlled Inspections, Special Inspections and any fees associated. Filing forms will be filled out by others, sealed and signed by the engineer for filing by others. Engineering support shall be limited to signing and sealing of MEP documents and filing forms (prepared and provided by others). Preparation of additional documents and drawings (including as-builts), as a result of DOB, expeditor and/or authority having jurisdiction including repackaging for additional sets shall be an additional service. Additional meetings for services noted above shall be billed as additional services.

KOHLER RONAN, LLC

37 Madison Avenue
New York, NY 10016
T (212) 695-2422
F (212) 695-2423

93 Lake Avenue
Danbury, CT 06810
T (203) 778-1017
F (203) 778-1018

kohlerronan.com

8. Additional work resulting in a change or deviation of scope as outlined in this document.
9. The preparation of work outside the standard of care.
10. Work relating to mold, asbestos or hazardous materials.
11. LEED Documentation
12. Energy Modeling
13. REVIT/BIM
14. Building commissioning.
15. Work related to submitting to local utility companies for energy rebates.

Reimbursable Expenses:

The following items will be invoiced at cost as a reimbursable expense exclusive of the fee.

1. Large format drawing plots shall be reimbursed at $9.00 per sheet.
2. Costs associated with mail and delivery services.
3. Costs associated with mileage. Mileage is reimbursable at the published IRS rate.

Additional Services:

1. Additional work resulting in a change or deviation of scope as outlined in this document.
2. This proposal is based on a sequential progression of the design phases with a maximum of 30 calendar days between phases. The Client shall compensate Kohler Ronan, LLC for expenses and fees incurred due to an extended schedule in excess of the stated maximum between design phases or as a result of the suspension and resumption of services.
3. This proposal assumes that the existing MEP infrastructure is adequate in capacity for the intended program, and information on the existing systems capacities and connection locations are available from the Owner. Work associated with the upgrade of existing systems can be provided as an additional service.
4. Existing MEP system elements that are discovered, that were not readily observable, requiring modifications to the documents will be an additional service.
5. Re-design required as a result of contractor material, or submittal substitutions.

KOHLER RONAN, LLC

77 Madison Avenue
New York, NY 10016
T (212) 695-2422
F (212) 695-2423

93 Lake Avenue
Danbury, CT 06810
T (203) 778-1017
F (203) 773-1018

kohlerronan.com

Rory S. Potter, PE
Craig F. Razza, PE
Joseph V. Lambo, PE
Steven V. Lambo, PE
Erik D. Bedeker, PE
Talya Spector, PE

6. Additional meetings, site visits, and contract administration in excess of work as outlined within this proposal.

7. Work associated with the preparation of manuals, operational/procedure guides, consultation associated with assisting the Owner in developing building dashboard protocols and graphics can be provided as an additional service if requested.

8. Preparation of as-built drawings.

9. Additional services will be invoiced at the hourly rates in effect at the time of service.

Special Conditions:

1. Attachment – A "Terms and Conditions" will apply and is part of this proposal. Signature on this proposal will signify acceptance of these "Terms and Conditions".

2. The quoted fee, scope of services, terms and conditions are considered valid for a period of 90 days from the date of issuance of this proposal.

Compensation:

The compensation for the scope of MEP services as listed within this proposal:

Construction Documents:	$14,500
Cost Estimator:	$ 1,320
	Total : $15,820

Terms For Payment:

Invoices will be sent on a monthly basis payable within 30 days of receipt. Monthly billing represents an estimated percentage of work completed.

Accepted: on behalf of Town of Weston

Authorized Signature _____ Date _____

General Notes:

1 The purpose of these drawings is to show the structural work associated with the basement alterations to Weston Town Hall located at 56 Norfield Road, Weston, CT.
2 The work shown on these drawings has been designed in accordance with the structural requirements of the 2018 Connecticut State **Building** Code which is the 2015 International Building Code as amended, altered or deleted by the provisions of the 2018 Connecticut amendments.
3 The structural components have been designed for the following loads:
A. Uniform live load:
Office buildings
Storage 100 psf
B. Wind design data:
Wind design for existing buildings:
Not required since the proposed alterations do not increase the demand-capacity ratio of any wind load carrying structural element by more than 10 percent cumulative since the original construction.
C. Earthquake design data:
Earthquake design for existing buildings:
Not required since the proposed alterations do not increase the demand-capacity ratio of any seismic load carrying structural element by more than 10 percent cumulative since the original construction.
D. Other loads:
Concentrated loads:
All floors except as noted (on 2-1/2 feet square) 2000 lbs
E. Special loads:
Retaining walls
Lateral equivalent fluid pressure 35 pcf
Seismic load (h = height of wall) 5.5 h^2
Vertical live load surcharge 100 psf
F. Existing buildings:
For existing structural elements carrying gravity loads, the proposed alterations do not increase the stress in any structural element by more than 5 percent nor do they decrease the strength of any structural element to less than required by the building code for new structures.
4 This structure has been designed to be self-supporting and stable after the work shown on these drawings has been completed. The stability of the structure prior to completion is solely the responsibility of the contractor. This responsibility extends to all related aspects of the construction activity including, but not limited to, erection methods, erection sequence, temporary bracing, forms, shoring, use of equipment, and similar construction procedures. Review of the construction by the engineer is for conformance with design aspects only, not to review the contractor's construction procedures. Lack of comment on the part of the engineer with regard to construction procedures is not to be interpreted as approval of those procedures.
5 Shoring note: The contractor is responsible for designing, providing and installing all temporary shoring that is required to support instabilities of existing structure during construction and due to the removal of existing supporting walls and existing framing members for installation of new framing and foundations. Shoring shall be fully installed and stable prior to removal of existing structural elements.
6 Jobsite safety and construction procedures are solely the responsibility of the contractor. Review of the construction by the engineer is for conformance with design aspects only, not to review the contractor's provisions for job site safety. Lack of comment by the engineer is not to be interpreted as approval of those aspects of work.
7 PDF digital files of all erection and detail shop drawings for steel reinforcing bars (concrete) and structural steel indicating the fabricator, manufacturer, finish, layout, and all accessories, must be submitted to and be checked by the contractor and subcontractor and bear the checker's initials before submission to the architect for review prior to fabrication. Fabrication and/or delivery to the site of components prior to receiving approved shop drawings shall be at the fabricator's own risk. After approval by the engineer, the deferred submittal documents shall be submitted to the building official for approval. Deferred submittal items shall not be installed until the deferred submittal documents have been approved by the building official.]
8 Testing and inspection of concrete, steel reinforcing bars (concrete and concrete masonry construction), structural steel and other work are described in the Quality Control Section of these notes. The contractor shall review the Quality Control Section and coordinate the scheduling of inspections with the testing and inspections agency and the engineer. Uninspected work that required inspections may be rejected solely on that basis.
9 If faulty construction procedures, or material, result in defective work that requires additional engineering time to devise corrective measures, professional fees may be charged to the contractor at the standard hourly rate of additional services. Such fees may be withheld from the general contractor's payment.
10 The contractor shall field verify existing conditions before proceeding with any work. The contractor shall field verify all dimensions noted "±" that are indicated on the drawings.
11 The contractor and subcontractors shall obtain the latest copies of approved plans and surveys and they shall familiarize themselves thoroughly with these plans before commencing any work.
12 Work shown as "Typical Details" apply throughout the project as required. Work shown as "Sections" shall be considered to apply for the same and similar conditions in the building.
13 Some details of the work are shown on the architectural drawings. A careful review and study of these details are necessary before the full scope of the work can be comprehended.
14 Do not scale drawings.

Codes and Standards References

1 Concrete:
Concrete work shall conform to all the requirements of ACI 301-10, "Specifications for Structural Concrete in Buildings" and ACI 318-14, "Building Code Requirements for Structural Concrete".
2 Structural steel:
Design, fabrication and erection of structural steel shall conform to the "Specification for Structural Steel for Buildings" as adopted on June 22, 2010, by the American Institute of Steel Construction (AISC) and the 14th Edition of the AISC Steel Construction Manual.

Special Notes to the Contractor

1 Contractors must review the work area prior to submitting construction proposals to determine the full scope of work involved. The drawings may not show all piping, conduits, equipment, and fixtures that may have to be re-routed or temporarily dislocated to perform the structural work shown.
2 The contractor shall verify all critical dimensions and elevations of the existing conditions before proceeding with any work.
3 The scheduling of all phases of the work, including any temporary service outages, shall be coordinated with the owner's designated on-site representative.
4 Before any work is started, the work area must be securely dust-proofed.
5 All finishes or materials damaged in the performance of the work shall be restored to original condition. Temporarily relocated utilities or equipment shall be put back in proper position.
6 The existing framing connections are assumed to be capable of supporting the additional loads imposed by this work, except where noted on plan. The contractor shall expose the connections noted on plan for verification by the engineer.
7 At the completion of the work, all temporary dust-proofing and construction debris is to be removed, and the area left in a clean and neat manner.

Demolition Notes

1 Definitions
A. Remove: Detach items from existing construction and legally dispose of them off-site unless indicated to be removed and reinstalled.
B. Remove and install: Detach items from existing construction, prepare for reuse, and reinstall where indicated.
C. Existing to remain: Existing items of construction that are not to be permanently removed and that are not otherwise indicated to be removed or removed and reinstalled.
2 Conduct demolition and remove debris to ensure minimum interference with roads, streets, walks, and other adjacent occupied and used facilities.
3 Do not close or obstruct streets, walks, or other adjacent occupied or used facilities without permission from owner.
4 Conduct demolition to prevent injury to people and damage to adjacent buildings and facilities to remain. Ensure safe passage of people around demolition area.
5 Provide and maintain shoring, bracing, and structural supports as required to preserve stability and prevent movement, settlement, or collapse of construction to remain.
6 Remove and transport debris in a manner that will prevent spillage on adjacent surfaces and areas.
7 Clean adjacent areas of dust, dirt, and debris caused by demolition. Return adjacent areas to condition existing before start of demolition.

Selective Demolition

1 General: Demolish and remove existing construction only to the extent required and as indicated. Use methods required to complete the work within limitations of governing regulations and as follows:
A. Proceed with selective demolition systematically, from higher to lower level. Complete selective demolition operations above each floor or tier before disturbing supporting members on the next lower level.
B. Neatly cut openings and holes plumb, square, and true to dimensions required. Use cutting methods least likely to damage construction to remain or adjoining construction. Use hand tools or small power tools designed for sawing or grinding, not hammering and chopping, to minimize disturbance of adjacent surfaces. Temporarily cover openings to remain.
C. Cut or drill from the exposed or finished side into concealed surfaces to avoid marring existing finished surfaces.
D. Remove decayed, vermin-infested, or otherwise dangerous or unsuitable materials and promptly dispose of off-site.
E. Dispose of demolished items and materials promptly.
2 Removed and reinstalled items:
A. Clean and repair items to functional condition adequate for intended reuse.
B. Pack or crate items after cleaning and repairing. Identify contents of containers.
C. Protect items from damage during transport and storage.
D. Reinstall items in locations indicated. Comply with installation requirements for new materials and equipment. Provide connections, supports, and miscellaneous materials necessary to make item functional for use indicated.
3 Existing items to remain:
A. Protect construction indicated to remain against damage and soiling during selective demolition.
B. When permitted by engineer, items may be removed to a suitable, protected storage location during selective demolition and cleaned and reinstalled in their original locations after selective demolition operations are complete.

Foundation notes:

1 The site preparation and earthwork within the perimeter of the proposed new structure shall include, as a minimum, the complete removal of all topsoil, organic and unsuitable fill materials. Proof compact the top of the remaining excavated surface. The removed soil shall be replaced with compacted structural fill where required for the support of foundations and slabs on grade.
2 The foundations have been designed to rest on inorganic, undisturbed soil or compacted granular fill having a presumptive bearing value of 3000 psf. Such bearing strata are anticipated at the bottom of footing elevations noted on the foundation plan. All bearing strata shall be reviewed by the engineer prior to placing concrete in order to verify the presumptive bearing value.
3 In areas requiring structural fill, the fill material shall be a uniformly graded mixture of sand and gravel weighing no less than 120 pcf dry density after compaction in place. This mixture shall be uniformly graded having no stone greater than 3 inches in any one dimension, with no more than 90 percent by weight passing a 1-1/2-inch sieve, and with less than 12 percent by weight, passing a no. 200 sieve. A soils testing lab, hired by the owner, shall test each on-site or borrow soil material proposed for backfill for classification according to ASTM D 2487 and for laboratory compaction curve according to ASTM D 1557. Uniformly moisten or aerate subgrade and each backfill layer before compaction to within 2 percent of optimum moisture content. The fill material shall be placed in maximum lifts of 8 inches in loose depth for material compacted by heavy compaction equipment, and in maximum lifts of 4 inches loose depth for material compacted by hand-operated tampers. Each lift shall be compacted with appropriate equipment to a minimum of 95 percent of its maximum density at or near optimum moisture. No lifts shall be placed when weather conditions are such that the moisture content of the fill cannot be properly controlled. In placing and compacting fill and backfill material, do not damage nor displace concrete work already in place by contact from compaction machinery, by subjecting it to overturning from heavy compacting loadings, or any other cause. Place fill against such concrete work at the same rate as the remainder of fill, compacting uniformly on both sides using hand - operated tampers. A soils testing lab, hired by the owner, shall test compaction of soils in place according to ASTM D 1556, ASTM D 2167, ASTM D 2922, and ASTM D 2937 as applicable. When test reports indicate that backfills have not achieved the degree of compaction specified, scarify and moisten or aerate, or remove and replace soil to depth required; recompact and retest until specified compaction is obtained.
4 The slab-on-grade sub-base shall be a crusher run stone free from soft disintegrated pieces, mud, dirt, or other injurious material. The material shall have no stone greater than 2 inches in any one dimension and with less than 10 percent by weight passing a No.100 sieve.
5 The bottom of exterior footings not on solid rock shall be at least 3' - 6" below finished grade.
6 All soil surrounding and under footings shall be protected from freezing and frost action during the course of construction.
7 Step footings where elevations change at a maximum slope of one vertical on two horizontal and place lower footings first.
8 Keep foundation excavations free of water at all times.
9 Use crushed stone backfill or controlled compacted fill or lean concrete (f'c=1500 psi) for over-excavation of footings.
10 Existing utilities: locate existing underground utilities in areas of excavation work. Provide adequate means of support and protection during earthwork operations.
11 Where footings are in close proximity to sub-surface piping bottom of footings shall be at least 8" below elevation of piping unless otherwise shown on the drawings.
12 Submittals to the engineer are required for structural fill and slab sub-base.

Concrete Notes:

1 All concrete work shall conform to all the requirements of ACI 301, "Specifications for Structural Concrete in Buildings" and ACI 318 "Building Code Requirements for Structural Concrete", as specified in the code reference section of these general notes.
2 Concrete shall be the specified weight and develop a minimum compressive strength in 28 days as follows:

Location	Weight	Minimum Strength	Maximum W/C Ratio (or slump where indicated)
Footings	Normal	3,000 psi	0.55
Walls and piers:			
Exterior	Normal	4,000 psi	0.45
Slabs-on-grade -interior	Normal	4,000 psi	0.45

3 All detailing fabrication, and erection of reinforcing bars, unless otherwise noted, must follow the latest ACI code and the latest ACI "Manual of Standard Practice for Detailing Reinforced Concrete Structures".
4 Concrete design mix will be submitted to the engineer for review, together with laboratory reports attesting that the mixes can attain the minimum strength required in accordance with ACI 301 indicated above.
5 Portland cement shall be Type I or Type II and conform to ASTM C 150.
6 Other cementitious material such as flyash or ground granulated blast- furnace slag may be blended with cement for use in the concrete mix. Flyash shall conform to ASTM C 618 and may replace cement if the following ranges for the 2 classes of flyash; Class C, 20 to 35%; Class F, 15 to 25%. Ground granulated blast- furnace slag shall conform to ASTM C 989 and may not exceed 50% of total weight of cementitious materials.
7 For normal weight concrete: coarse aggregate shall be 3/4" and conform to ASTM C 33. Fine aggregate shall be manufactured or natural sand from the same source for the entire project and shall conform to ASTM C 33.
8 No admixtures are permitted without the engineer's written permission other than entrained air. Concrete exposed to the weather, such as that used in foundation walls, shall contain 5% +/- 1 1/2% entrained air. Concrete exposed to the weather and to de-icing compounds shall contain 6% +/- 1 1/2% entrained air. Do not use air entrainment admixture for interior normal weight concrete slabs.
9 Limit water-soluble, chloride-ion content in hardened concrete to the following percent by weight of cement: 1.00 for reinforced concrete that will be dry and protected from moisture, 0.30 for reinforced concrete that will exposed to moisture but not exposed to chlorides, 0.15 for reinforced concrete exposed to moisture and chlorides from deicing chemicals and salt/seawater, and 0.06 for prestressed (post-tensioned) concrete.
10 Reinforcing steel shall conform to ASTM A 615, Grade 60.
11 Welded wire fabric shall conform to ASTM A 1064 with a minimum yield strength of 65 ksi. Lap one mesh size at sides and ends, and wire together.
12 Vapor retarder shall be Stego Wrap (15 mil) vapor retarder by Stego Industries LLC and shall have a water vapor permeance after conditioning (ASTM E 1745, Paragraphs 7.1.2 – 7.1.5) that is less than 0.01 perms and meets the requirements of Class A. Place as per ASTM E 1643 and manufacturer's written instructions.
13 The following concrete cover shall be provided for reinforcement:

Location	Cover (inches)
Concrete cast against and Permanently exposed to earth	3
Concrete exposed to earth or weather:	
#6 through #18 bars	2
#5 bar and smaller	1 1/2
Concrete not exposed to weather or in contact with ground:	
Slabs, walls, joists:	
#11 bar and smaller	3/4
Beams, columns	
Primary reinforcement, ties, stirrups, spirals	1 1/2

14 The conveyance, placement and protection of the concrete shall conform to the requirements of ACI 318, indicated above, and ACI 304R, "Guide for Measuring, Mixing, Transporting and Placing Concrete". Mechanical vibrators are to be used to consolidate the freshly cast concrete around the reinforcing and against form surfaces and to prevent the formation of air or stone pockets, honeycombing, pitting or planes of weakness. However, care must be used to avoid over vibration that can lead to aggregate segregation.
15 No welding of reinforcing will be permitted.
16 All lap splices shall be Class B, in accordance with ACI 318 indicated above.
17 Concrete piers: Place concrete piers and walls together. Set pier reinforcing and set wall reinforcing through pier vertical bars. Provide dowels with standard hook from footing at all piers. Size and quantity of dowels to match vertical pier reinforcing (Class "B" splice).
18 The contractor shall be responsible for limiting pours to minimize shrinkage cracking. In general, walls shall not be poured in continuous lengths exceeding **[30]** feet without providing construction joints or control joints. The location and configuration of joints exposed to view shall be coordinated with the architect.
19 The installation of slabs shall conform to the requirements of ACI 302.1R, "Guide to Concrete Floor and Slab Construction". Interior finish slab surfaces are to have a steel trowel finish. Exterior slab surfaces are to have a broom finish unless specified on the architectural drawings.
20 Expansion and isolation-joints:
Filler strips: ASTM D 1751, asphalt-saturated cellulosic fiber or ASTM D 1752, cork or self expanding cork
Sealant at top of joint: Sika's Sikaflex 2c SL poly urethane elastomeric sealant. Provide cap to separate sealant from filler.
21 The curing and protection of concrete shall conform to the requirements of ACI 318 and ACI 308R, "Guide to Curing Concrete". Concrete slabs shall be protected from loss of surface moisture for not less than 7 days using a curing compound conforming to ASTM C 309 or constantly wetted burlap. Curing compounds shall be compatible with any intended flooring overlay. Do not install finish flooring until slab has adequately dried per the flooring manufacturer's specifications.
22 Cold weather concrete placement: If cold weather concreting conditions exist as defined by a period of more than three days when the average outdoor temperature, (high + low)/2, is less than 40 deg. F. the procedures outlined in ACI 306.1, "Standard Specification for Cold Weather Concreting" and ACI 306R, "Guide to Cold Weather Concreting" shall be utilized.
23 Hot weather concrete placement: Maintain concrete temperature below 90 deg. F. at time of placement and comply with ACI 301 and ACI 305R, "Guide to Hot Weather Concreting".
24 Accurately position, support, and secure reinforcement and anchors against displacement. Locate and support reinforcement with bar supports to maintain minimum concrete cover. Do not tack weld crossing reinforcing bars. Provide bar supports as follows:

A. Bolsters, chairs, spacers, and other devices for spacing, supporting, and fastening reinforcing bars and welded wire reinforcement in place. Do not "wet stick" reinforcement or anchors. Manufacture bar supports from steel wire, plastic, or precast concrete according to CRSI's "Manual of Standard Practice," of greater compressive strength than concrete.

25 Sizes and locations of all required embedded items, such as anchor bolts, piping sleeves etc., for all trades shall be coordinated by the general contractor with other trades.
26 Submittals to the engineer are required for concrete mix designs, cement, reinforcing bars, admixtures, and aggregates.

Connections to Existing Masonry or Hardened Concrete:

1 All proprietary anchoring systems (expansion, adhesive anchoring systems, etc.) to be installed into hardened concrete and masonry elements are to be installed in strict accordance with the manufacturer's instructions for drilling and preparation of holes, for spacing and edge distance requirements, and for the utilization of supplemental components for the anchoring systems such as screen tubes, doweling adhesives, etc.
2 Connections to hardened concrete shall be made with anchors conforming to ACI 318, as specified in the code reference section of these general notes, for cracked concrete, and Chapter 19 of the state building code indicated at the beginning of these general notes.
A. Mechanical anchors shall be either
i. Hilti "Kwik Bolt TZ" expansion anchor.
ii. Hilti "Kwik HUS-EZ" screw anchor (use only in permanently dry, interior non-corrosive environments)
iii. Simpson "Strong Bolt 2" expansion anchor
iv. Simpson "Titen HD" screw ancho, zinc-plated or galvanized (use only in permanently dry, interior non-corrosive environments)

Size, embedment, spacing and edge distance of anchors shall be as indicated on the drawings.
B. Adhesive anchor rods or reinforcing bars shall be installed in rotary hammered drilled holes with carbide drill bits using one of the following adhesive anchoring systems:
i. Hilti "HIT-HY 200 safe set or Hilti "HIT-RE-500 V3" adhesive anchoring system with Hilti "HAS" ASTM F1554, Grade 36 anchor rods.
ii. Reinforcing bars shall conform to the requirements of the Concrete General Notes.

Adhesive for reinforcing bars and anchors shall have been tested in accordance with ACI 355.4 "Qualification of Post-Installed Adhesive Anchors in Concrete" and ICC-ES (ICC Evaluation Service) "Acceptance Criteria for Post-Installed Adhesive Anchors in Concrete Elements" (AC308) for cracked concrete and seismic applications.

Adhesive bond design strength is based upon concrete that has cured at least 21 days with a minimum compressive strength of 2,500 psi and an in-service temperature in accordance with ACI 355.4 Temperature Category B.

Installation method shall be in accordance with the Manufacturer's Printed Installation Instructions (MPII
Installation of adhesive anchors horizontally or upwardly inclined to support sustained tension loads shall be performed by personnel certified by the ACI/CRSI Adhesive Anchor Installer Certification program or equal.

Unless otherwise noted on the drawings, embed anchor rods and reinforcing bars into drilled holes a minimum of 9 anchor diameters, with a minimum edge distance of 4 inches, measured from the edge of the concrete to the centerline of the anchor/reinforcing bar. Increased embedment depths or edge distances may be required at certain locations, see plans and details.
3 Connections to grout filled concrete masonry shall be made with either 1) Hilti "HAS" ASTM F1554 Grade 36 anchor rods using Hilti "HIT HY270" masonry adhesive anchoring systems or 2) Simpson "RFB" ASTM F1554 Grade 36 anchor rods using Simpson "Set-XP" masonry adhesive anchoring system or 3) ASTM F1554 Grade 36 anchor rods using Dewalt AC100+ Gold masonry adhesive anchoring system.
A. Unless otherwise noted on the drawings, embed anchor rods into drilled holes a minimum of 9 anchor diameters, with a minimum edge distance of 4 measured from the edge of the masonry to the centerline of the anchor. Increased embedment depths or edge distances may be required at certain locations, see plans and details.
4 Connections to hollow concrete or clay brick masonry shall be made with either 1) Hilti "HAS" ASTM F1554 Grade 36 anchor rods using Hilti "HIT HY270" masonry adhesive anchoring system with "HIT-SC" composite screen tubes or 2) Simpson "RFB" ASTM F1554 Grade 36 anchor rods using Simpson "SET-XP" masonry adhesive anchoring system with Simpson "Opti-mesh" plastic screen tubes or 3) ASTM F1554 Grade 36 anchor rods using Dewalt AC100+ Gold masonry adhesive anchoring system with composite screen tubes.
A. For anchors in hollow concrete masonry, embed anchor rods into drilled holes a minimum of 2 inches, with a minimum edge distance of 4 inches, unless otherwise noted, measured from the edge of the masonry to the centerline of the anchor. Increased embedment depths or edge distances may be required at certain locations, see plans and details.
B. For anchors in hollow clay brick masonry, embed anchor rods into drilled holes a minimum of 3 1/2 inches, with a minimum edge distance of 4 inches, unless otherwise noted, measured from the edge of the masonry to the centerline of the anchor. Increased embedment depths or edge distances may be required at certain locations, see plans and details.

Structural Steel Notes:

1 Design fabrication and erection of structural steel shall conform to the American Institute of Steel Construction's "Specification for Structural Steel for Buildings", as specified in the code reference section of these general notes.
2 Materials:

Wide flange shapes:	ASTM A 992 Grade 50
American standard shapes, angles, Plates and bars:	ASTM A 36
Structural steel tubing, Structural steel pipe:	ASTM A 53, Grade B (Fy=35 ksi)
Bolts	ASTM F3125, Grade A 325
Anchor rods	ASTM F 1554, Grade 36
Welding electrode	ASTM E 70xx, low hydrogen
Shear connectors	3/4" diam. ASTM A 108

3 All welding shall conform to American Welding Society's AWS D1.1 "Structural Welding Code-Steel" code for arc and gas welding and be performed by a certified welder in accordance with A.W.S. standards.
4 High strength bolts: install high-strength bolts according to Research Council on Structural Connections' (RCSC's) "Specification for Structural Joints Using High-Strength Bolts" for type of bolt and type of joint specified.
Joint type: Snug tightened unless otherwise noted.
5 Grout shall be nonmetallic, shrinkage-resistant grout conforming to ASTM C 1107, Grade B or C, factory-packaged, nonmetallic aggregate grout, noncorrosive, nonstaining, mixed with water to consistency suitable for application and a 30-minute working time.
6 Structural steel shall be cleaned in accordance with the Steel Structures Painting Council Specification SP 3 for Power Tool Cleaning (except for steel exposed to weather).
7 Provide bitumastic protection coating for all structural steel below grade.
8 Continuous members, where indicated on the drawings, shall require either 1) the member to be furnished as one piece, or 2) if individual pieces are to be provided, then they shall be connected by either welding or bolting to develop the full strength of the continuous member.
9 Split cantilevers for steel beams shall be designed for the full moment capacity of the beam unless otherwise noted.
10 Unless otherwise noted, at cantilever beam connection to top of columns, provide welded 3/4" cap plate with (4)-3/4" diameter A 325 bolts on beam gage. Provide 1/2" minimum fitted stiffeners welded at both sides of beam web. Locate stiffeners over column flange on cantilever side.
11 For miscellaneous steel, see architectural drawings.
12 Existing steel surfaces to receive field welds shall be thoroughly cleaned and free from paint, rust, grease, etc.
13 Submittals to the engineer are required for certificates of compliance for structural steel, bolts, nuts, washers, and weld filler material prior to the fabrication of any steel
14 At the completion of fabrication, the fabricator shall submit a certificate of compliance stating that the work was performed in accordance with the approved contract documents, as required by Section 1704.2 of the building code indicated at the beginning of these General Notes.

Quality Control

1 General:
A The owner shall employ an independent testing and inspection agency to perform the tests and inspections indicated under this Quality Control Section. Reports shall be submitted to the architect, engineer and owner in a timely manner.
B The contractor shall notify in a timely manner the testing and inspection agency and the engineer to schedule field inspections.
2 Concrete:
A Reinforcing: inspect 50% of installed reinforcing bars.
B Cast-in-place anchors: inspect 50% of anchors for materials, size, positioning, spacing, edge distance and embedment.
C Post-installed mechanical and adhesive anchors and dowels: inspect 100% of mechanical anchors and 75% of adhesive anchor for materials, size, positioning, spacing, edge distance and embedment. Inspect drilled holes (for proper preparation, size, depth and cleaning) and anchor and dowel installation for compliance with manufacturer's requirements.
D Conduct strength tests in accordance with the following procedures: (A strength test consists of four concrete cylinders.)
I Make one strength test for each 50 cubic yards or fraction thereof from each mix design of concrete placed in any one day, except that in no case shall a given mix design be represented by less than five tests.
II Secure composite samples in accordance with the "Standard Practice for Sampling Freshly Mixed Concrete" (ASTM C 172). Each strength test shall be obtained from a different batch of concrete on a representative, truly random basis. When pumping or pneumatic equipment is used, samples shall be taken at the discharge end.
III Mold four specimens from each sample in accordance with the "Standard Practice for Making and Curing Concrete Test Specimens in the Field" (ASTM C 31), and cure under standard moisture and temperature conditions, in accordance with Section 7(a) and 7(b) of the above ASTM method.
IV Determine slump of the concrete sample for each strength test and whenever consistency of concrete appears to vary using the "Standard Test Method for Slump of Hydraulic-Cement Concrete" (ASTM C 143).
V Determine air content of normal weight concrete sample for each strength test in accordance with either the "Standard Test Method for Air Content of Freshly Mixed Concrete by the Pressure Method " (ASTM C 231), or the "Standard Test Method for Air Content of Freshly Mixed Concrete by the Volumetric Method" (ASTM C 173).
VI Test three specimens: one at seven days, and two at 28 days in accordance with the "Standard Test Method for Compressive Strength of Cylindrical Concrete Specimens" (ASTM C 39). The 28 day test result shall be the average of the two specimens. If the average of the two specimens is less than the required strength, test the fourth specimen at 45 days. When high early strength is required, two specimens shall be tested at seven days.
E Curing and protection: Periodically review curing temperatures and protection techniques. Also inspect hot and cold weather procedures as applicable to be in accordance with ACI 305R (hot weather) and ACI 306.1/306R (cold weather).

3 Structural steel:
A Fillet welds: visually inspect 50% of welds by an A.W.S. certified weld inspector.
B High strength bearing type bolt connections: Inspect 50% of bolted connections for required size, location and number of bolts and for contact of plies. The minimum bolt tension, method of tightening and method of inspecting installed bolts shall be as specified in AISC "Specifications for Structural Joints Using ASTM A 325 or A 490 bolts". Use of a calibrated torque wrench on a previously tightened bolt is not an acceptable method of inspection.

No.	Date	Description
	6/2/2022	Design Development

Issue/Revisions

The DiSalvo Engineering Group
Structural Engineers
83 Wooster Heights Road I Suite 200
Lee Farm Corporate Park I Danbury, CT 06810
(203) 490-4140 I www.tdeg.com

Seal

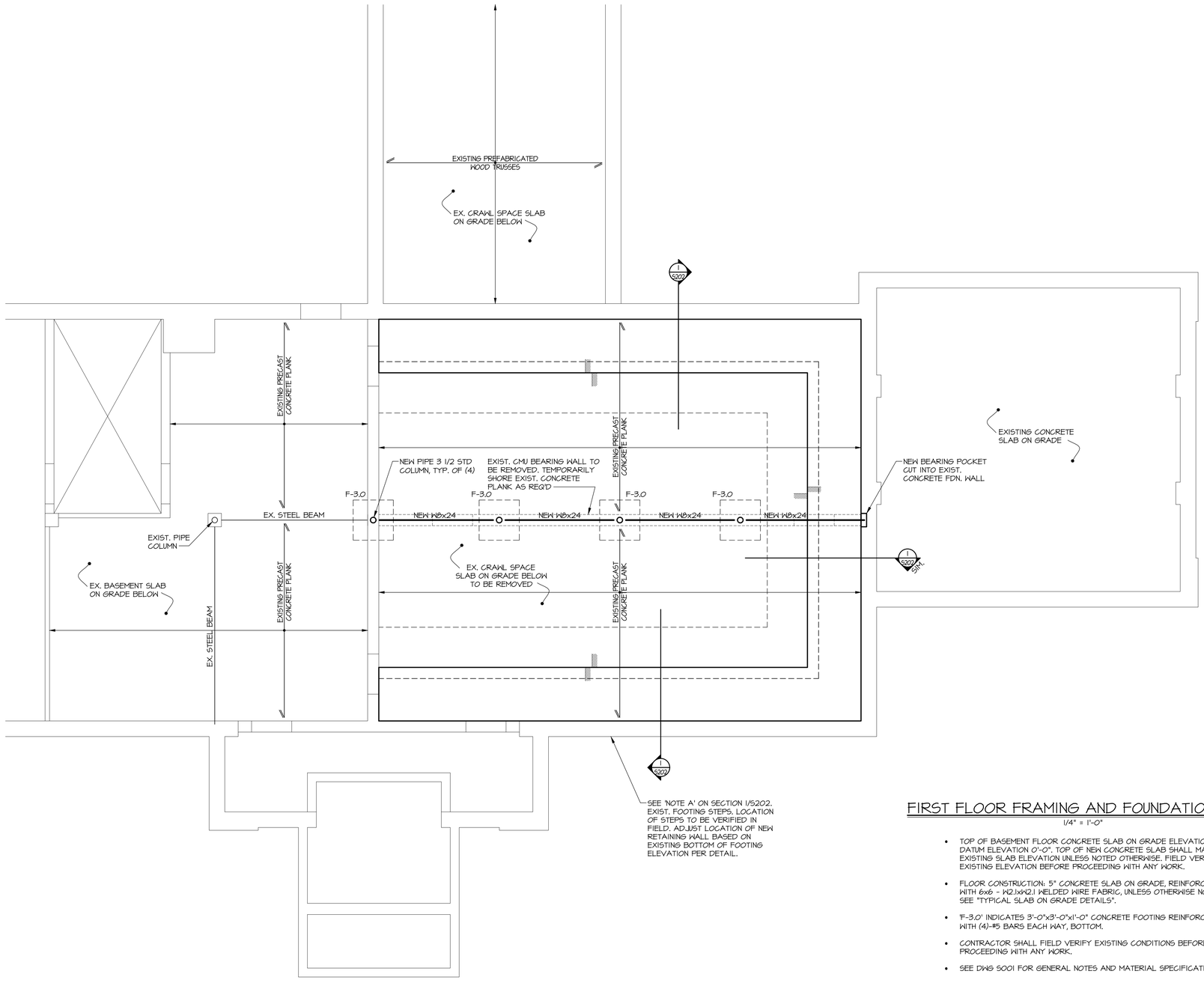
Weston Town Hall Basement Alterations

56 Norfield Road
Weston, CT 06883

Drawing Title
General Notes and Material Specifications

Scale	AS NOTED
Date	June 2, 2022
Drawn By	JEH
Checked By	BDR
Job Number	21273.00

Drawing Number
S001



FIRST FLOOR FRAMING AND FOUNDATION PLAN

1/4" = 1'-0"

NORTH:

- TOP OF BASEMENT FLOOR CONCRETE SLAB ON GRADE ELEVATION = DATUM ELEVATION 0'-0". TOP OF NEW CONCRETE SLAB SHALL MATCH EXISTING SLAB ELEVATION UNLESS NOTED OTHERWISE. FIELD VERIFY EXISTING ELEVATION BEFORE PROCEEDING WITH ANY WORK.
- FLOOR CONSTRUCTION: 5" CONCRETE SLAB ON GRADE, REINFORCED WITH 6x6 - W2.1xW2.1 WELDED WIRE FABRIC, UNLESS OTHERWISE NOTED. SEE "TYPICAL SLAB ON GRADE DETAILS".
- 'F-3.0' INDICATES 3'-0"x3'-0"x1'-0" CONCRETE FOOTING REINFORCED WITH (4)-#5 BARS EACH WAY, BOTTOM.
- CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS BEFORE PROCEEDING WITH ANY WORK.
- SEE DWG S001 FOR GENERAL NOTES AND MATERIAL SPECIFICATIONS.

No.	Date	Description
	6/2/2022	Design Development

Issue/Revisions

The DiSalvo Engineering Group
Structural Engineers
83 Wooster Heights Road | Suite 200
Lee Farm Corporate Park | Danbury, CT 06810
(203) 490-4140 | www.tdeg.com

Seal

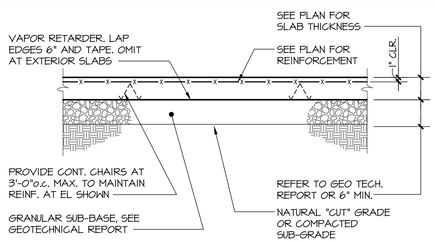
Weston Town Hall Basement Alterations

56 Norfield Road
Weston, CT 06883

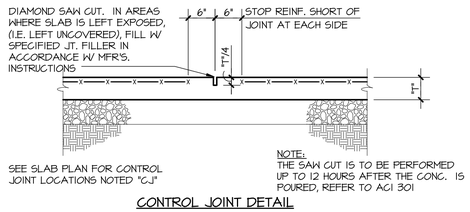
Drawing Title
First Floor Framing and Foundation Plan

Scale	AS NOTED
Date	June 2, 2022
Drawn By	JEH
Checked By	BDR
Job Number	21273.00

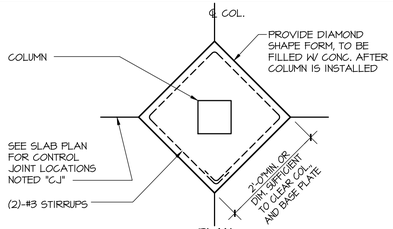
Drawing Number
S101



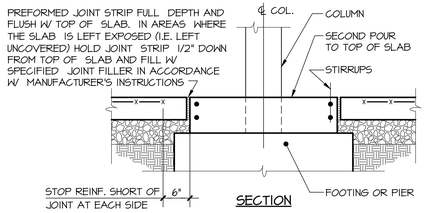
SLAB ON GRADE DETAIL



CONTROL JOINT DETAIL



PLAN



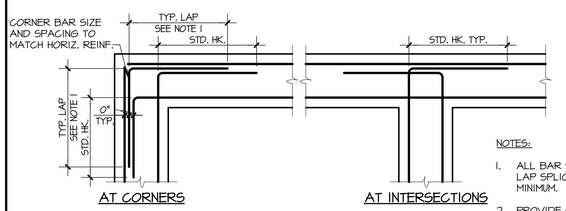
SECTION

SLAB AT COLUMN DETAIL

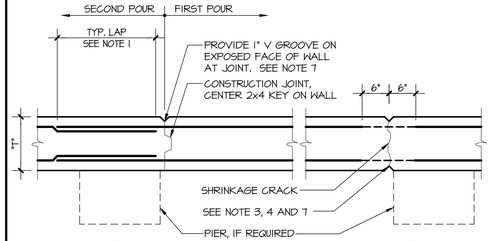
TYPICAL SLAB ON GRADE DETAILS

NO SCALE

1 S201



AT CORNERS AT INTERSECTIONS



CONSTRUCTION JOINT CONTROL JOINT

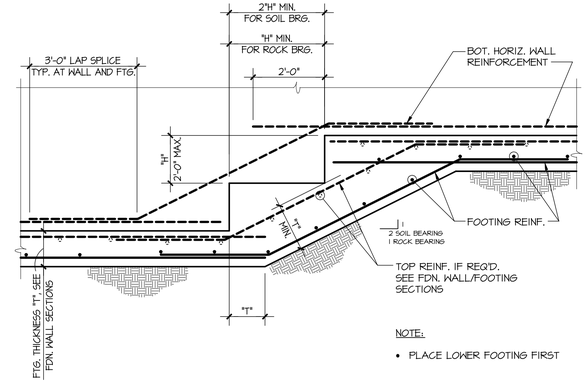
NOTES:

1. ALL BAR SPLICES SHALL BE CLASS "B" LAP SPLICES WITH 56 BAR DIAMETERS MINIMUM.
2. PROVIDE CONSTRUCTION JOINT AT END OF EACH DAY'S POUR. WALLS SHALL NOT BE POURED IN CONTINUOUS LENGTHS EXCEEDING 30 FEET WITHOUT PROVIDING CONSTRUCTION JOINTS OR CONTROL JOINTS.
3. PROVIDE CONSTRUCTION/CONTROL JOINTS AT FACE OF PIERS UNLESS OTHERWISE NOTED ON PLAN.
4. AT CONTROL JOINT, PROVIDE BEVEL JOINT "T"/10 INTO WALL SURFACE BOTH SIDES. STOP ALTERNATING HORIZONTAL REINFORCING BARS 6" SHORT OF CONTROL JOINT.
5. SEE APPROPRIATE SECTION FOR
6. WALL REINFORCEMENT.
7. VERTICAL REINF. AND/OR SUPPORT BARS NOT SHOWN FOR CLARITY.
8. PROVIDE CAULKING AT JOINT IF REQUIRED. SEE SPECIFICATIONS.

TYPICAL CONCRETE WALL CONSTRUCTION DETAIL

3/4" = 1'-0"

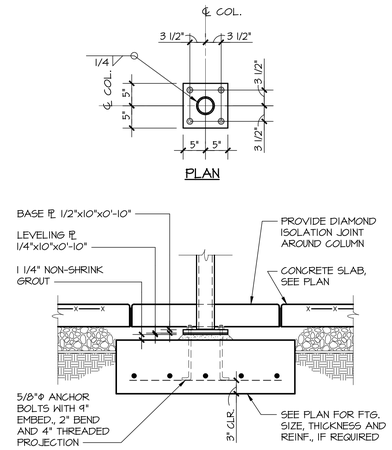
2 S201



TYPICAL STEPPED FOOTING DETAIL

3/4" = 1'-0"

3 S201



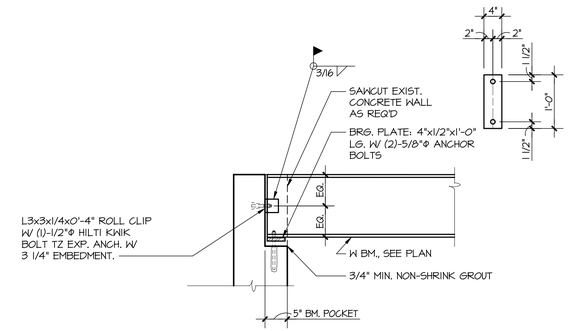
PLAN

SECTION

TYPICAL INTERIOR COLUMN BASE DETAIL AT NEW SLAB

3/4" = 1'-0"

4 S201



TYPICAL BEAM BEARING POCKET DETAIL

3/4" = 1'-0"

5 S201

No.	Date	Description
	6/2/2022	Design Development

Issue/Revisions

The DiSalvo Engineering Group
Structural Engineers
83 Wooster Heights Road | Suite 200
Lee Farm Corporate Park | Danbury, CT 06810
(203) 490-4140 | www.tdeg.com

Seal

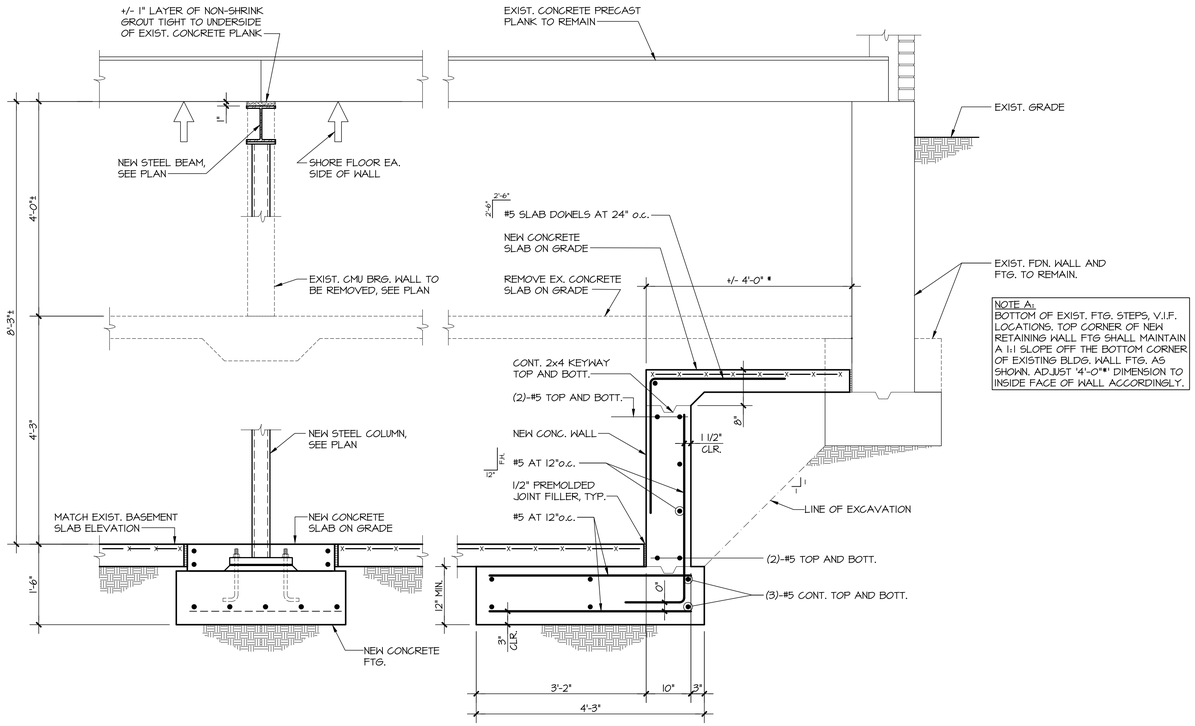
Weston Town Hall Basement Alterations

56 Norfield Road
Weston, CT 06883

Drawing Title
Typical Details

Scale	AS NOTED
Date	June 2, 2022
Drawn By	JEH
Checked By	BDR
Job Number	21273.00

Drawing Number
S201



SECTION

3/4" = 1'-0"

1 / S202

No.	Date	Description
	6/2/2022	Design Development

Issue/Revisions

The DiSalvo Engineering Group
Structural Engineers
83 Wooster Heights Road | Suite 200
Lee Farm Corporate Park | Danbury, CT 06810
(203) 490-4140 | www.tdeg.com

Seal

Weston Town Hall Basement Alterations

56 Norfield Road
Weston, CT 06883

Drawing Title

Sections

Scale	AS NOTED
Date	June 2, 2022
Drawn By	JEH
Checked By	BDR
Job Number	21273.00

Drawing Number

S202

Item 3

Jonathan Luiz <jluiz@westonct.gov>

Senior Center Bathroom Renovations

1 message

Jonathan Luiz <jluiz@westonct.gov> Thu, May 5, 2022 at 2:52 PM
To: Samantha Nestor <snestor@westonct.gov>

Hi Sam,

In FY 21-22, the Town budgeted $28,000 for renovations to a bathroom at the Senior Center. The estimated cost for that project is $32,276.79 because the architect's fee was $1,797.50 (invoice attached) and the quote from a contractor is $30,479.29 (see attached). If we add a contingency of 10%, then the project's estimated cost would be $35,504.47. Given the fact that we only have $28,000 to spend, the shortfall for this project would be $7,504.47

In FY 22-23, the Town would budget another $20,000 for renovations to two bathrooms at the Senior Center. The contractor has provided a quote for one bathroom totalling $ $22,123.47 and a quote for another bathroom totaling $ $21,228.60 . The two quotes are attached. Since we would only have $20,000 to spend on this project, the shortfall would be $23,352.07.

Please consider allocating $31,000 from the ARPA funds to go towards these three bathroom renovation projects at the Sr. Center.

Sincerely,
Jonathan Luiz
Weston Town Administrator

4 attachments

- **$1797,50 Architect Invoice.pdf**
  53K
- **$21,228.60 bathroom quote.pdf**
  101K
- **$22,123.47 bathroom quote.pdf**
  102K
- **$30,479.29 bathroom quote.pdf**
  160K

(1st)

Capitol Region Council of Governments

Price Proposal Review Summary - Category

GORDIAN®

Job Order Contract

Date:	March 31, 2022
Contract Number:	SW GC CRCOG ezIQC
Job Order Number:	103227.00
Job Order Title:	Weston Senior Center Bathroom Reno
Contractor:	BMP Construction Inc.
Proposal Value:	$30,479.29
Proposal Submitted:	03/30/2022

Category - 01: General Conditions:	**$1,149.44**
Category - 02: Demolition:	**$979.01**
Category - 03: Carpentry:	**$6,084.92**
Category - 05: Concrete Work:	**$4,051.79**
Category - 06: Plumbing:	**$4,444.00**
Category - 07: HVAC:	**$1,886.47**
Category - 08: Electrical:	**$3,032.91**
Category - 09: Ceramic Tile:	**$4,650.61**
Category - 10: Doors, Frames and Hardware:	**$2,004.67**
Category - 11: Bathroom Hardware:	**$1,162.62**
Category - 11: Painting:	**$1,032.85**
Proposal Total	**$30,479.29**

This proposal total represents the correct total for the proposal. Any discrepancy between line totals, sub-totals and the proposal total is due to rounding of the line totals and sub-totals.

The Percent of NPP on this Proposal: **0.00%**

Capitol Region Council of Governments

Price Proposal Review Summary - Category

GORDIAN®

Job Order Contract

Date:	May 04, 2022
Contract Number:	SW GC CRCOG ezIQC
Job Order Number:	104653.00
Job Order Title:	Weston Sr. Center Add""l Bathroom Reno
Contractor:	BMP Construction Inc.
Proposal Value:	$22,123.47
Proposal Submitted:	05/04/2022

Category - 01: General Conditions:	**$1,521.70**
Category - 02: Demolition:	**$1,005.99**
Category - 03: Carpentry:	**$2,608.29**
Category - 05: Masonry:	**$1,582.99**
Category - 06: Plumbing:	**$5,551.84**
Category - 07: HVAC:	**$1,886.47**
Category - 08: Electrical:	**$1,198.06**
Category - 09: Ceramic Tile:	**$4,724.68**
Category - 11: Bathroom Hardware:	**$1,010.60**
Category - 11: Painting:	**$1,032.85**
Proposal Total	**$22,123.47**

This proposal total represents the correct total for the proposal. Any discrepancy between line totals, sub-totals and the proposal total is due to rounding of the line totals and sub-totals.

The Percent of NPP on this Proposal: **0.00%**

Gym

Capitol Region Council of Governments

Price Proposal Review Summary - Category

GORDIAN®

Job Order Contract

Date:	May 04, 2022
Contract Number:	SW GC CRCOG ezIQC
Job Order Number:	104915.00
Job Order Title:	Weston Gym Restroom Reno
Contractor:	BMP Construction Inc.
Proposal Value:	$21,228.60
Proposal Submitted:	05/04/2022

Category - 01: General Conditions:	**$1,521.70**
Category - 02: Demolition:	**$1,005.99**
Category - 03: Carpentry:	**$2,608.29**
Category - 05: Masonry:	**$1,582.99**
Category - 06: Plumbing:	**$4,656.97**
Category - 07: HVAC:	**$1,886.47**
Category - 08: Electrical:	**$1,198.06**
Category - 09: Ceramic Tile:	**$4,724.68**
Category - 11: Bathroom Hardware:	**$1,010.60**
Category - 11: Painting:	**$1,032.85**
Proposal Total	**$21,228.60**

This proposal total represents the correct total for the proposal. Any discrepancy between line totals, sub-totals and the proposal total is due to rounding of the line totals and sub-totals.

The Percent of NPP on this Proposal: **0.00%**

Item 4

Weston Building Committee
Special Meeting Minutes
May 17, 2022 at 7:00 PM
Meeting was held in the Town Hall Commission Room

1) Call to order: Chairman Richard Wolf called the meeting to order at 7 pm. With him in the Commission Room were Committee members David Coprio, Jack Davidoff, and Edmond Warchick. Committee members on the phone were Megan Loucas and Joe Stromwall. Town Administrator Jonathan Luiz was present.
2) Update on High School Old Gym air handler replacement project: School Facilities Director Mike DelMastro provided an update on the project. Mr. Davidoff made several suggestions to Mr. DelMastro, who said he would try to incorporate them into the project.
3) Update on Intermediate School Window Sill Repair Project: Mr. Luiz shared a correspondence from the general contractor about the projected start time.
4) Discussion/decision on Town Hall basement records room project: Mr. Luiz provided an update on the project, explaining that DiSalvo Engineering is under contract. Mr. Wolf requested that a meeting be coordinated with Mr. Luiz, Mr. Davidoff and representatives from Kohler Ronan concerning the company's proposal for HVAC work.
5) Discussion/decision on Senior Center bathroom renovations: Mr. Wolf summarized the situation. He requested that the quotes be shared with the soon-to-be-appointed Committee member Al Fazi for his review and comment.
6) Approval of Minutes of prior meeting: Mr. Stromwall moved and Mr. Coprio seconded to approve the minutes from the March 22, 2022 meeting. The motion carried unanimously.
7) Adjournment: Ms. Loucas moved and Mr. Warchick seconded to adjourn. The motion carried unanimously.